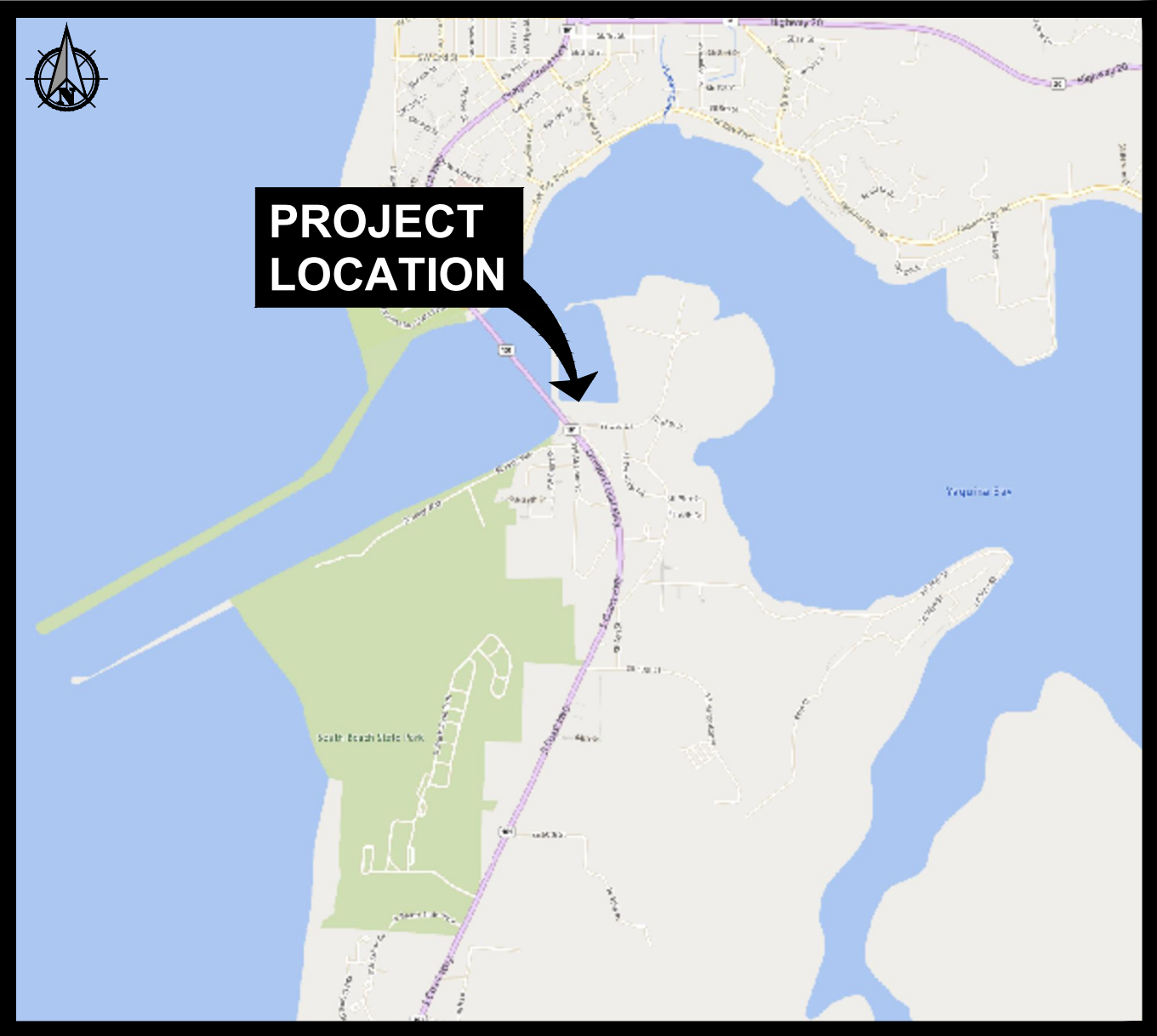
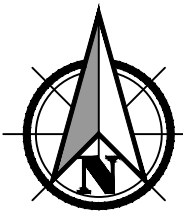


ROGUE BREWERY SEAWALL REPAIR

2320 OSU DRIVE, NEWPORT, OREGON 97365

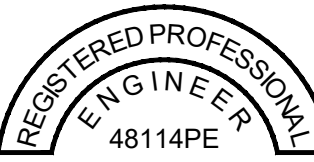


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Sheet Number	Sheet Title
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S-103	SEAWALL SECTION VIEW
S-104	DETAILS

COVER SHEET FOR:

ROGUE BREWERY SEAWALL REPAIR
A SITE LOCATED IN THE CITY OF NEWPORT, OREGON



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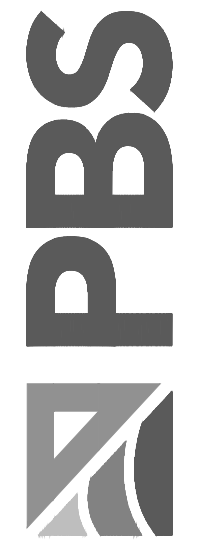
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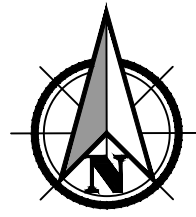
SHEET 1 OF 11

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NOTE: LEAD TIME SHOULD BE PLANNED FOR ANY ACTIVITY THAT MAY DISRUPT THE BREWERY PRODUCTION. CONTRACTOR TO CONSULT WITH OWNER FOR SCHEDULING DETAILS.

TEMPORARY FACILITIES PLAN
SCALE: NTS

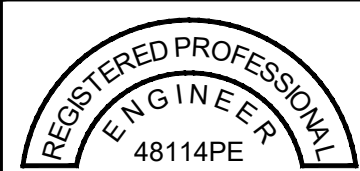
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SITE PREP AND DEMO PLAN FOR:
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SHEET **2** OF **11**

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PROJECT DESCRIPTION

THE WORK CONSISTS OF SEAWALL BACKFILL STABILIZATION USING POLYMER INJECTION TECHNIQUES, INSTALLATION OF DRAINS TO MINIMIZE DIFFERENTIAL HYDROSTATIC HEAD, REPAIR OF CORRODED PILING BY WELDING ON NEW STEEL PLATES TO RESTORE THE STRUCTURAL SECTION, AND COATING OF EXPOSED STEEL ELEMENTS TO INCREASE CORROSION PROTECTION.

GENERAL

- THE STRUCTURAL DRAWINGS ARE A PORTION OF THE CONTRACT DOCUMENTS AND ARE INTENDED TO BE USED IN CONJUNCTION WITH OTHER PROJECT DOCUMENTS. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE REQUIREMENTS FROM THE ENTIRE SET OF CONTRACT DOCUMENTS (INCLUDING THE PROJECT SPECIFICATIONS) INTO THEIR WORK.
- THESE GENERAL NOTES SUPPLEMENT THE PROJECT SPECIFICATIONS. REFER TO THE PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
- NOTES AND DETAILS ON THE STRUCTURAL DRAWINGS SHALL TAKE PRECEDENCE OVER THE GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS.
- DETAILS ON THESE PLANS ARE INTENDED TO DEPICT THE GENERAL CONSTRUCTION METHODS FOR THIS STRUCTURE. CONNECTIONS, DETAILS AND CONDITIONS NOT SPECIFICALLY SHOWN THAT ARE SIMILAR TO THOSE THAT ARE SPECIFIED SHALL BE ASSUMED ONE AND THE SAME. IF QUESTIONS REGARDING THE APPLICATION OF DETAILS ARE ENCOUNTERED, NOTIFY THE ENGINEER OF RECORD FOR CLARIFICATION IN A TIMELY MANNER PRIOR TO BID OPENING.

CODE REQUIREMENTS

- CONFORM TO THE 2022 OREGON STRUCTURAL SPECIALTY CODE (OSSC), BASED UPON THE 2021 INTERNATIONAL BUILDING CODE (IBC).
- ALL REFERENCE TO OTHER CODES AND STANDARDS (ACI, ASTM, ETC.) SHALL BE FOR THE EDITIONS NOTED IN CHAPTER 35 OF THE IBC.

TEMPORARY CONDITIONS

- THE STRUCTURE HAS BEEN DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT REQUIRED AS A RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES.
- CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.

EXISTING CONDITIONS

- ALL EXISTING CONDITIONS, DIMENSIONS, AND ELEVATIONS SHALL BE FIELD VERIFIED. THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD OF ANY SIGNIFICANT DISCREPANCIES FROM CONDITIONS SHOWN ON THE DRAWINGS.

DESIGN CRITERIA

- DESIGN WAS BASED ON THE STRENGTH AND DEFLECTION CRITERIA OF THE IBC. IN ADDITION TO THE DEAD LOADS, THE FOLLOWING LOADS AND ALLOWANCES WERE USED FOR DESIGN, WITH LIVE LOADS (LL) REDUCED IN ACCORDANCE WITH THE IBC.

DESIGN CRITERIA

GEOTECHNICAL CRITERIA

DESIGN BASED ON GRI GEOTECHNICAL MEMORANDUM DATED
GEOTECHNICAL REPORT BY: OCT. 7, 2021

LIVE LOAD CRITERIA

FLOOR LIVE LOADS	UNIFORM LOAD (PSF)
STORAGE (LIGHT)	125

STRUCTURAL OBSERVATIONS

CONSTRUCTION PHASE	OBSERVATION BY SER	COMMENTS
AS REQUIRED TO ADDRESS STRUCTURAL ISSUES	X	REF. FOOTNOTE A, B

STRUCTURAL OBSERVATION

- THE STRUCTURAL ENGINEER OF RECORD (SER) WILL PERFORM STRUCTURAL OBSERVATIONS BASED ON THE REQUIREMENTS OF THE IBC AT THE STAGES OF CONSTRUCTION LISTED BELOW. THE CONTRACTOR SHALL PROVIDE SUFFICIENT NOTICE AND ACCESS FOR THE SER TO PERFORM THESE OBSERVATIONS
 - STRUCTURAL OBSERVATIONS ARE INTENDED TO VERIFY GENERAL CONFORMANCE WITH THE STRUCTURAL DRAWINGS. SPECIAL INSPECTIONS AND TESTING ARE STILL REQUIRED.
 - A FIELD REPORT WILL BE SUBMITTED TO THE BUILDING DEPARTMENT FOLLOWING EACH VISIT.

SPECIAL INSPECTION AND TESTING

- SPECIAL INSPECTION WILL BE PROVIDED BY THE OWNER BASED ON THE REQUIREMENTS OF THE IBC AS SUMMARIZED IN THE SPECIAL INSPECTION AND TESTING TABLES. THE CONTRACTOR SHALL PROVIDE SUFFICIENT NOTICE AND ACCESS FOR THE SPECIAL INSPECTOR TO PERFORM THESE INSPECTIONS.

SUBMITTALS

- SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD PRIOR TO THE FABRICATION AND CONSTRUCTION OF ALL STRUCTURAL ITEMS INCLUDING THE FOLLOWING:

SUBMITTALS

ITEM	SUBMITTAL (A, D)	DEFERRED SUBMITTAL (B, D)	COMMENTS
STRUCTURAL STEEL	X		
STEEL WELDING PROCEDURES	X		
WELDER CERTIFICATIONS	X		
			A. IF THE SHOP DRAWINGS DIFFER FROM OR ADD TO THE DESIGN OF THE STRUCTURAL DRAWINGS, THEY SHALL BEAR THE SEAL AND SIGNATURE OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED. ANY MODIFICATIONS TO THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD AND ARE SUBJECT TO REVIEW AND ACCEPTANCE BY THE STRUCTURAL ENGINEER OF RECORD.
			B. DESIGN DRAWINGS, SHOP DRAWINGS, AND CALCULATIONS FOR THE DESIGN AND FABRICATION OF ITEMS THAT ARE DESIGNED BY OTHERS SHALL BEAR THE SEAL AND SIGNATURE OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED. CALCULATIONS SHALL BE INCLUDED FOR ALL CONNECTIONS TO THE STRUCTURE CONSIDERING LOCALIZED EFFECTS ON STRUCTURAL ELEMENTS INDUCED BY THE CONNECTION LOADS. DESIGN SHALL BE BASED UPON THE REQUIREMENTS OF THE IBC AND AS NOTED UNDER "DESIGN CRITERIA."
			C. THE CONTRACTOR SHALL COORDINATE THE VERTICAL AND SEISMIC RESTRAINTS OF MECHANICAL, ELECTRICAL, AND PLUMBING EQUIPMENT, MACHINERY, AND ASSOCIATED PIPING WITH THE STRUCTURE. CONNECTIONS TO THE STRUCTURE SHALL CONFORM TO ASCE 7-16 CHAPTER 13 AND BE DESIGNED BY AN PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED.
			D. FIELD ENGINEERED DETAILS DEVELOPED BY THE CONTRACTOR THAT DIFFER FROM, OR ADD TO, THE STRUCTURAL DRAWINGS SHALL BEAR THE SEAL AND SIGNATURE OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED AND SHALL BE SUBMITTED PRIOR TO CONSTRUCTION. ANY SUCH DETAILS ARE SUBJECT TO REVIEW AND ACCEPTANCE BY THE STRUCTURAL ENGINEER OF RECORD.
			E. THE DELEGATED DESIGNER SHALL, IN CONJUNCTION WITH THE GENERAL CONTRACTOR, COORDINATE THE VERTICAL AND SEISMIC RESTRAINTS AND LOADING OF EQUIPMENT WITH THE STRUCTURE. CONNECTIONS TO THE STRUCTURE SHALL CONFORM TO IBC AND ASCE 7 AND BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED.

CODES AND STANDARDS

- INTERNATIONAL BUILDING CODE (IBC) 2021
- ASCE 7-16 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
- ACI 318-19 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
- AISC 360-16 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS
- AWS D1.1, 2020 STRUCTURAL WELDING CODE – STEEL

STRUCTURAL STEEL

- STEEL CONSTRUCTION SHALL CONFORM TO AISC 360-16 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS AND AISC 341-10 SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS.

- STEEL SHALL CONFORM TO THE FOLLOWING:

WIDE FLANGE	ASTM A992	Fy = 50 KSI
ANGLE & CHANNEL (UNO)	ASTM A36	Fy = 36 KSI
PLATE AND BAR STOCK	ASTM A572 GRADE 50	Fy = 50 KSI
PIPE	ASTM A53 GRADE B	Fy = 35 KSI
STRUCTURAL TUBE (HSS)	ASTM A500 GRADE B	Fy = 46 KSI
ROUND STRUCTURAL TUBE (HSS)	ASTM A500 GRADE B	Fy = 42 KSI
OTHER STEEL UNO	ASTM A572	Fy = 50 KSI

- WORK SHALL BE IN ACCORDANCE WITH AISC 303, AS MODIFIED PER THE PROJECT SPECIFICATIONS.

- HOLES IN STEEL CUT IN THE FIELD SHALL BE DRILLED, NOT BURNED.

- STEEL BEAMS ARE EQUALLY SPACED BETWEEN DIMENSION POINTS UNO.

- ITEMS EMBEDDED IN CONCRETE OR MASONRY AND EXPOSED TO EARTH OR WEATHER SHALL BE HOT-DIPPED GALVANIZED (UNLESS NOTED OTHERWISE):

A. CONCRETE & MASONRY ACCESSORIES OF FERROUS MATERIAL INCLUDING: INSERTS, EMBEDS, AND BRACKETS.

B. PIPE GUARDS, BOLLARDS AND HARDWARE FOR SAME.

C. WHERE NOTED AS "GALV".

- LOCATIONS OF EMBEDDED ITEMS SHALL BE COORDINATED WITH VENDOR'S SHOP DRAWINGS.

- THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE SELECTION OF OPTIONAL DETAILS SHOWN ON THE DRAWINGS.

- THE CONTRACTOR SHALL BE RESPONSIBLE FOR ERECTION AIDS THAT INCLUDE, BUT ARE NOT LIMITED TO, ERECTION ANGLES, LIFT HOLES, AND OTHER AIDS. REMOVE ERECTION AIDS, INCLUDING ERECTION BOLTS. PLUG WELD HOLES AND GRIND SMOOTH.

- MEANS AND METHODS OF ERECTION ARE THE SOLE RESPONSIBLTY OF THE CONTRACTOR AND CONNECTIONS AND DETAILS DO NOT NECESSARILY SHOW ERECTION AIDS REQUIRED.

STRUCTURAL STEEL WELDING

- WELDING SHALL BE DONE BY CERTIFIED WELDERS AND IN ACCORDANCE WITH AWS D1.1 AND D1.8.

- WELD SIZES SHOWN ON THE DRAWINGS ARE MINIMUM AND EFFECTIVE SIZES. INCREASE WELD SIZE TO AWS/AISC MINIMUM SIZES AS REQUIRED FOR CONNECTION MATERIAL. THE MINIMUM WELD SIZE SHALL BE 3/16 INCHES (EFFECTIVE).

- FIELD WELDING SYMBOLS HAVE NOT NECESSARILY BEEN INDICATED ON THE DRAWINGS. WHERE NO FIELD WELDING IS DETAILED ON THE DRAWINGS, IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE THE USE OF SHOP AND FIELD WELDS.

- PARTIAL JOINT PENETRATION (PJP) GROOVE WELDS SHOWN ON THE DRAWINGS REFER TO EFFECTIVE THROAT THICKNESS. COMPLETE JOINT PENETRATION (CJP) WELDS SHALL BE ULTRASONICALLY TESTED (UT) UPON COMPLETION OF THE CONNECTION, EXCEPT PLATE LESS THAN OR EQUAL TO 5/16 INCH THICK SHALL BE MAGNETIC PARTICLE TESTED (MT).

- WELDS SHALL BE MADE USING LOW HYDROGEN ELECTRODES WITH TENSILE STRENGTH PER AWS D1.1 AND D1.8 (MINIMUM 70 KSI).

- WELDING SHALL BE PERFORMED IN STRICT ADHERENCE TO A WRITTEN WELDING PROCEDURE SPECIFICATION (WPS) PER AWS D1.1 AND D1.8. WELDING PROCEDURES SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW BEFORE STARTING FABRICATION OR ERECTION. COPIES OF THE WPS SHALL BE ON SITE AND AVAILABLE TO WELDERS AND THE SPECIAL INSPECTOR.

NON-SHRINK GROUT

NON-SHRINK GROUT SHALL BE CEMENTITIOUS HIGH PRECISION, NATURAL AGGREGATE, NON-METALLIC, NON-STAINING, NON-SHRINKABLE GROUT CONFORMING TO ASTM C1107 AND CRD-C 621, CORPS OF ENGINEERS "SPECIFICATIONS FOR NON-SHRINK GROUT". GROUT SHALL HAVE A SPECIFIED MINIMUM COMPRESSIVE STRENGTH OF 8,000 PSI AT 5 DAYS. PRE-GROUTING OF BASE PLATES SHALL NOT BE PERMITTED.

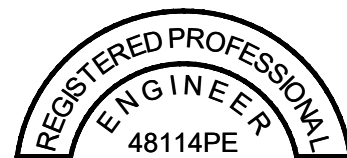
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GENERAL NOTES FOR:
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STATEMENT OF STRUCTURAL SPECIAL INSPECTION

1. SPECIAL INSPECTION SHALL CONFORM TO SPECIFICATION SECTION 01 40 00 AND CHAPTER 17 OF THE IBC.
2. SPECIAL INSPECTIONS AND ASSOCIATED TESTING SHALL BE PERFORMED BY AN INSPECTION AND TESTING AGENCY MEETING THE REQUIREMENTS OF SPECIFICATION SECTION 01 40 00.

TABLE 1705.3					
REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION					
	TYPE	CONTINUOUS	PERIODIC	REFERENCED STANDARD	IBC REFERENCE
4.	INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE.				
	a. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS.	X	-	ACI 318: 17.8.2.4	-
	b. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.A.	-	X	ACI 318: 17.8.2	-

AISC 360-16 CHAPTER N AND AISC 341-16 CHAPTER J - WELDING

QC = QUALITY CONTROL, TO BE PERFORMED BY THE FABRICATOR'S OR ERECTOR'S QUALITY CONTROL INSPECTOR
QA = QUALITY ASSURANCE, TO BE PERFORMED AT THE FABRICATORS PLANT OR IN THE FIELD BY THE QUALITY ASSURANCE INSPECTOR
O = OBSERVE ITEMS RANDOMLY
P = PERFORM TASKS FOR EACH ELEMENT

TABLE N 5.4-1				
INSPECTION TASKS PRIOR TO WELDING				
VERIFICATIONS AND INSPECTION	QC	QA	AWS REF.	
1. WELDER QUALIFICATION RECORDS AND CONTINUITY RECORDS	P	O		
2. WPS AVAILABLE	P	P	6.3	
3. MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	P	P	6.2	
4. MATERIAL IDENTIFICATION (TYPE/GRADE)	O	O	6.2	
5. WELDER IDENTIFICATION SYSTEM, SEE NOTE 1	O	O	6.4	
7. CONFIGURATION AND FINISH OF ACCESS HOLES	O	O	6.5.2, 5.16	
8. FIT-UP OF FILLET WELDS				
A. DIMENSIONS (ALIGNMENT, GAPS AT ROOT)	P/O SEE NOTE 2	O	5.21.1	
B. CLEANLINESS (CONDITION OF STEEL SURFACES)			5.14	
C. TACKING (TACK WELD QUALITY AND LOCATION)			5.17	
9. CHECK WELDING EQUIPMENT	O	-	6.2, 5.10	

- NOTE 1: THE FABRICATOR OR ERECTOR, AS APPLICABLE, SHALL MAINTAIN A SYSTEM BY WHICH A WELDER WHO HAS WELDED A JOINT OR MEMBER CAN BE IDENTIFIED. STAMPS, IF USED, SHALL BE THE LOW-STRESS TYPE.
- NOTE 2: FOLLOWING PERFORMANCE OF THIS INSPECTION TASK FOR TEN WELDS TO BE MADE BY A GIVEN WELDER, WITH THE WELDER DEMONSTRATING UNDERSTANDING OF REQUIREMENTS AND POSSESSION OF SKILLS AND TOOLS TO VERIFY THESE ITEMS, THE 'PERFORM' DESIGNATION OF THIS TASK SHALL BE REDUCED TO 'OBSERVE', AND THE WELDER SHALL PERFORM THIS TASK. SHOULD THE INSPECTOR DETERMINE THAT THE WELDER HAS DISCONTINUED PERFORMANCE OF THIS TASK, THE TASK SHALL BE RETURNED TO 'PERFORM' UNTIL SUCH TIME AS THE INSPECTOR HAS RE-ESTABLISHED ADEQUATE ASSURANCE THAT THE WELDER WILL PERFORM THE INSPECTION TASKS LISTED.

TABLE N 5.4-2				
INSPECTION TASKS DURING WELDING				
VERIFICATIONS AND INSPECTION	QC	QA	AWS REF.	
1. CONTROL AND HANDLING OF WELDING CONSUMABLES			6.2	
A. PACKAGING	O	O	5.3.1	
B. EXPOSURE CONTROL	O	O	5.3.2, 5.3.3	
2. NO WELDING OVER CRACKED WELDS	O	O	5.17	
3. ENVIRONMENTAL CONDITIONS				
A. WIND SPEED WITHIN LIMITS	O	O	5.11.1	
B. PRECIPITATION AND TEMPERATURE			5.11.2.	
4. WPS FOLLOWED				
A. SETTINGS ON WELDING EQUIPMENT	O	O	6.3.3, 6.5.2, 5.5, 5.20	
B. TRAVEL SPEED				
C. SELECTED WELDING MATERIALS				
D. SHIELDING GAS TYPE/ FLOW RATE				
E. PREHEAT APPLIED			5.6, 5.7	
F. INTERPASS TEMPERATURE MAINTAINED (MIN/MAX.)				
G. PROPER POSITION (F, V, H, OH)				
H. INTERMIX OF FILLER MATERIALS AVOIDED UNLESS APPROVED			AISC 341-16	
5. WELDING TECHNIQUES				
A. INTERPASS AND FINAL CLEANING	O	O	6.5.2, 6.5.3,	
B. EACH PASS WITHIN PROFILE LIMITATIONS	O	O	5.23, 5.29.1	
C. EACH PASS MEETS QUALITY REQUIREMENTS	O	O		
6. PLACEMENT AND INSTALLATION OF STEEL HEADED STUD ANCHORS	P	P		
7. USE OF QUALIFIED WELDERS	O	O	AISC 341-16	

TABLE N 5.4-3				
INSPECTION TASKS AFTER WELDING				
VERIFICATIONS AND INSPECTION	QC	QA	AWS REF.	
1. WELDS CLEANED	O	O	5.29.1	
2. SIZE, LENGTH AND LOCATION OF WELDS	P	P	6.5.1	
3. WELDS MEET VISUAL ACCEPTANCE CRITERIA				
A. CRACK PROHIBITION	P	P	6.5.3	
B. WELD/BASE-METAL FUSION			TABLE 6.1 (1)	
C. CRATER CROSS SECTION			TABLE 6.1 (2)	
D. WELD PROFILES			TABLE 6.1 (3)	
E. WELD SIZE			TABLE 6.1 (4), 5.24	
F. UNDERCUT			TABLE 6.1 (6)	
G. POROSITY			TABLE 6.1 (7)	
4. ARC STRIKES	P	P	5.28	
5. K-AREA, SEE NOTE 3	P	P	NOT ADDRESSED	
6. WELD ACCESS HOLES IN ROLLED HEAVY SHAPES AND BUILT-UP HEAVY SHAPES, SEE NOTE 4	P	P	5.16, 6.5.2	
7. BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	P	P	5.9, 5.30	
8. REPAIR ACTIVITIES	P	P	6.5.3, 5.25	
9. DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	P	P	6.5.4, 6.5.5	
10. NO PROHIBITED WELDS HAVE BEEN ADDED WITHOUT THE APPROVAL OF THE EOR	O	O		
11. PLACEMENT OF REINFORCING OR CONTOUR FILLET WELDS (IF REQUIRED)	P	P	AISC 341-16	

- NOTE 3: WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OR STIFFENERS HAS BEEN PERFORMED IN THE K-AREA, VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3 INCHES (75MM) OF THE WELD.
- NOTE 4: AFTER ROLLED HEAVY SHAPES (AISC 360-16 SECT. A3.1c) AND BUILT-UP HEAVY SHAPES (AISC 360-16 SECT. A3.1d) ARE WELDED, VISUALLY INSPECT THE WELD ACCESS FOR CRACKS.

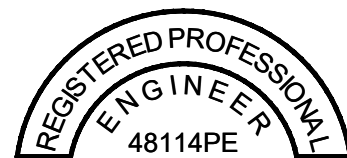
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SPECIAL INSPECTIONS FOR:
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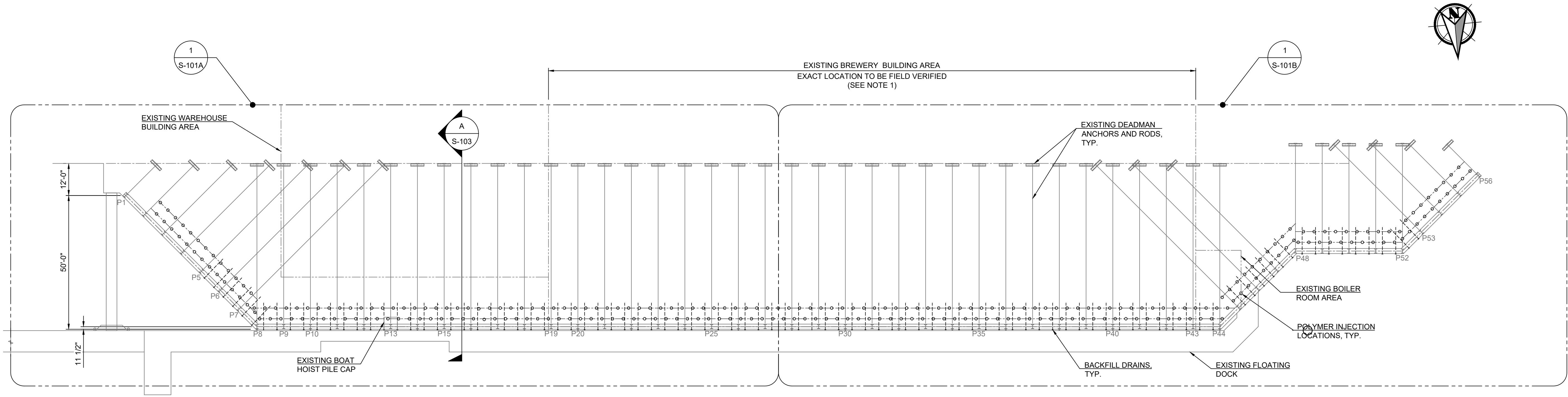
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SHEET **4** OF **11**

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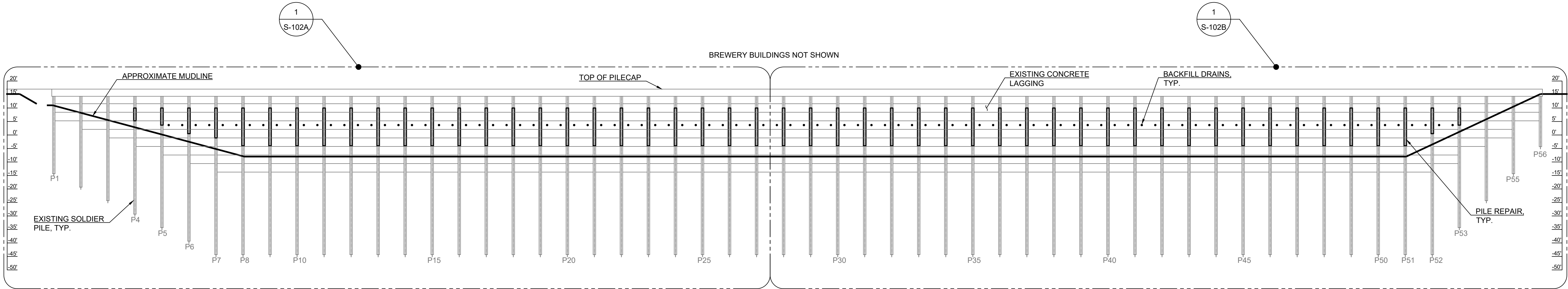


SEAWALL OVERALL PLAN

SCALE: 1" = 20'

NOTES:

1. EXISTING BUILDING STRUCTURAL COMPONENTS AND UTILITIES ARE NOT SHOWN IN PLAN.



SEAWALL OVERALL ELEVATION

SCALE: 1" = 20'

LEGEND:

Px = SOLDIER PILE NUMBER

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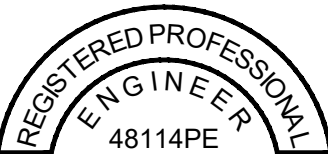
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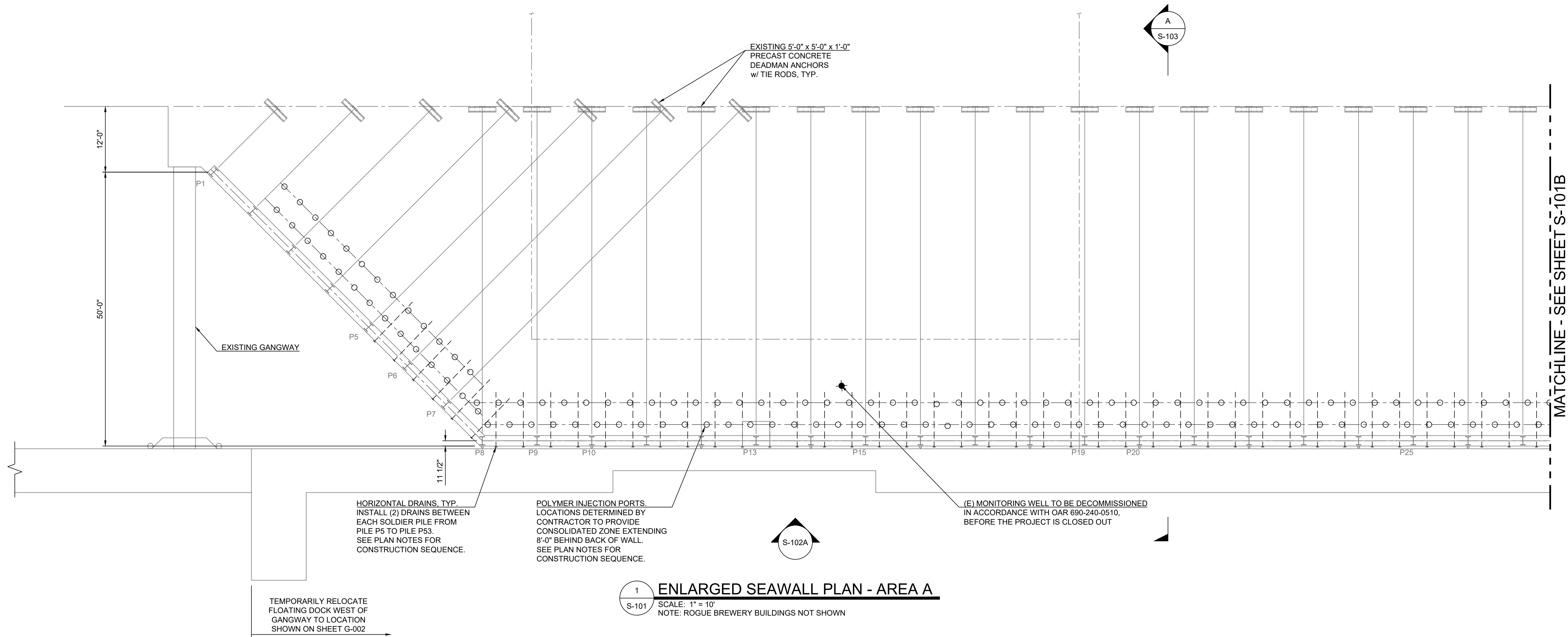
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1
S-101
ENLARGED SEAWALL PLAN - AREA A
SCALE: 1" = 10'
NOTE: ROGUE BREWERY BUILDINGS NOT SHOWN

CONSTRUCTION SEQUENCE NOTES:

1. BEGIN ALL WORK AT WEST END OF SEAWALL AND PROGRESS TOWARD THE EAST.
2. PERFORM POLYMER INJECTION SO AS TO NOT CREATE A CONTINUOUS, UNDRAINED BULKHEAD.
3. HORIZONTAL DRAINS MUST BE INSTALLED IN CURED POLYMER IN A TYPICAL BAY (E.G., BETWEEN P49 AND P50) PRIOR TO INSTALLING POLYMER IN AN ADJACENT BAY.
4. POLYMER MAY BE INSTALLED IN ALTERNATING BAYS PRIOR TO INSTALLING DRAINS.
5. SUBMIT POLYMER INJECTION AND DRAIN INSTALLATION PLAN PRIOR TO MOBILIZATION.

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ENLARGED SEAWALL PLAN - AREA A FOR:
ROGUE BREWERY SEAWALL REPAIR
A SITE LOCATED IN THE CITY OF NEWPORT, OREGON



Know what's below.
Call before you dig.



RENEWS: 06/30/24

DESIGNED:
LG

CHECKED:
HAW

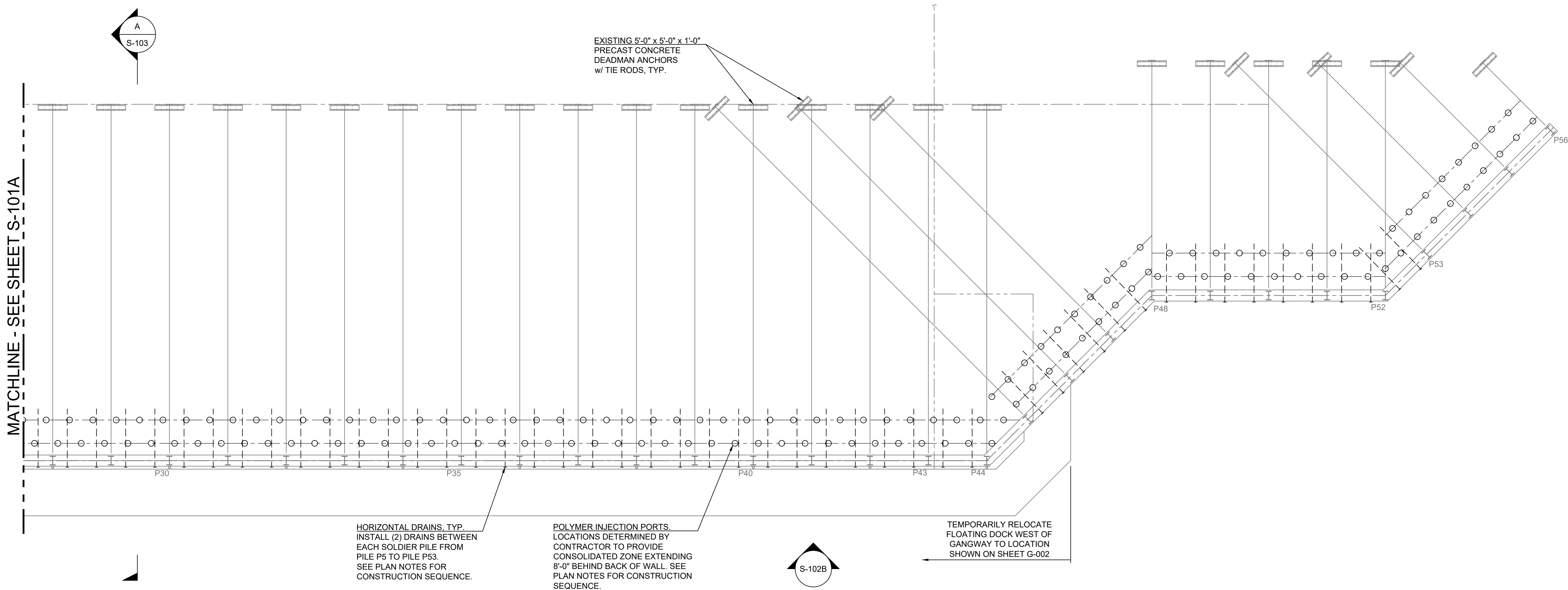
DECEMBER 2023
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SHEET ID

S-101A

SHEET 6 OF 11

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1
S-101
ENLARGED SEAWALL PLAN - AREA B
SCALE: 1" = 10'
NOTE: ROGUE BREWERY BUILDINGS NOT SHOWN

- CONSTRUCTION SEQUENCE NOTES:
1. BEGIN ALL WORK AT WEST END OF SEAWALL AND PROGRESS TOWARD THE EAST.
 2. PERFORM POLYMER INJECTION SO AS TO NOT CREATE A CONTINUOUS, UNDRAINED BULKHEAD.
 3. HORIZONTAL DRAINS MUST BE INSTALLED IN CURED POLYMER IN A TYPICAL BAY (E.G., BETWEEN P49 AND P50) PRIOR TO INSTALLING POLYMER IN AN ADJACENT BAY.
 4. POLYMER MAY BE INSTALLED IN ALTERNATING BAYS PRIOR TO INSTALLING DRAINS.
 5. SUBMIT POLYMER INJECTION AND DRAIN INSTALLATION PLAN PRIOR TO MOBILIZATION.

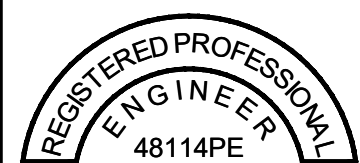
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ENLARGED SEAWALL PLAN - AREA B FOR:
ROGUE BREWERY SEAWALL REPAIR
A SITE LOCATED IN THE CITY OF NEWPORT, OREGON



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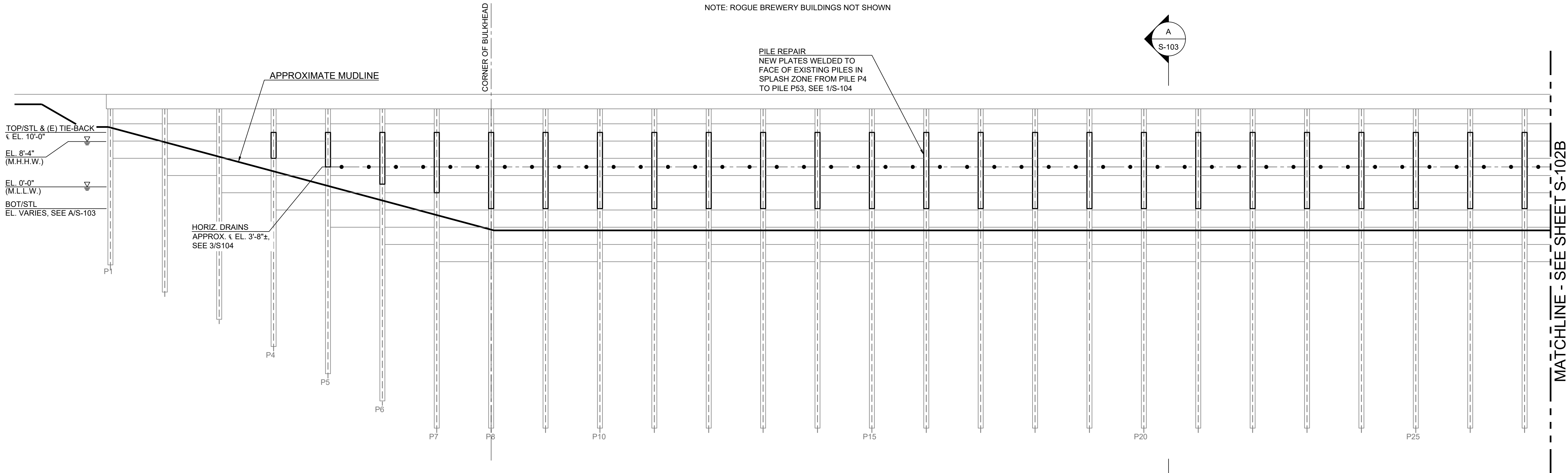
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SHEET ID

S-101B

SHEET 7 OF 11

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1
S-101 ENLARGED SEAWALL ELEVATION - AREA A
SCALE: 1" = 10'

TIDAL DATA		
ELEVATION	M.L.L.W. (FT)	NAVD88 (FT)
BASE FLOOD	13.74	13.00
H.M.T.	12.25	11.51
M.H.H.W.	8.34	7.60
M.H.W.	7.65	6.91
M.T.L.	4.52	3.78
M.S.L.	4.46	3.72
D.T.L.	4.17	3.43
M.L.W.	1.38	0.64
NAVD88	0.74	0.00
M.L.L.W.	0.00	-0.74
MIN.	-3.52	-4.26

BASE FLOOD	100-YEAR FLOOD
H.M.T.	HIGHEST MEASURED TIDE (12-11-1969)
M.H.H.W.	MEAN HIGHER-HIGH WATER
M.H.W.	MEAN HIGH WATER
M.T.L.	MEAN TIDE LEVEL
M.S.L.	MEAN SEA LEVEL
D.T.L.	MEAN DIURNAL TIDE LEVEL
M.L.W.	MEAN LOW WATER
NAVD88	NORTH AMERICAN VERTICAL DATUM OF 1988
M.L.L.W.	MEAN LOWER-LOW WATER
MIN.	LOWEST OBSERVED WATER LEVEL (06-01-1973)

EXISTING PILING APPROXIMATE DATA		
PILE	TIP ELEV	LENGTH
1 & 55	-14'-4"	30'
2	-19'-4"	35'
3 & 54	-24'-4"	40'
4	-29'-4"	45'
5 & 53	-34'-4"	50'
6	-39'-4"	55'
7 TO 52 INCL.	-44'-4"	60'
56	-4'-4"	20'

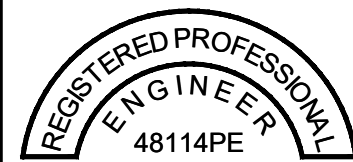
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ENLARGED SEAWALL ELEVATION - AREA A FOR:
ROGUE BREWERY SEAWALL REPAIR
A SITE LOCATED IN THE CITY OF NEWPORT, OREGON



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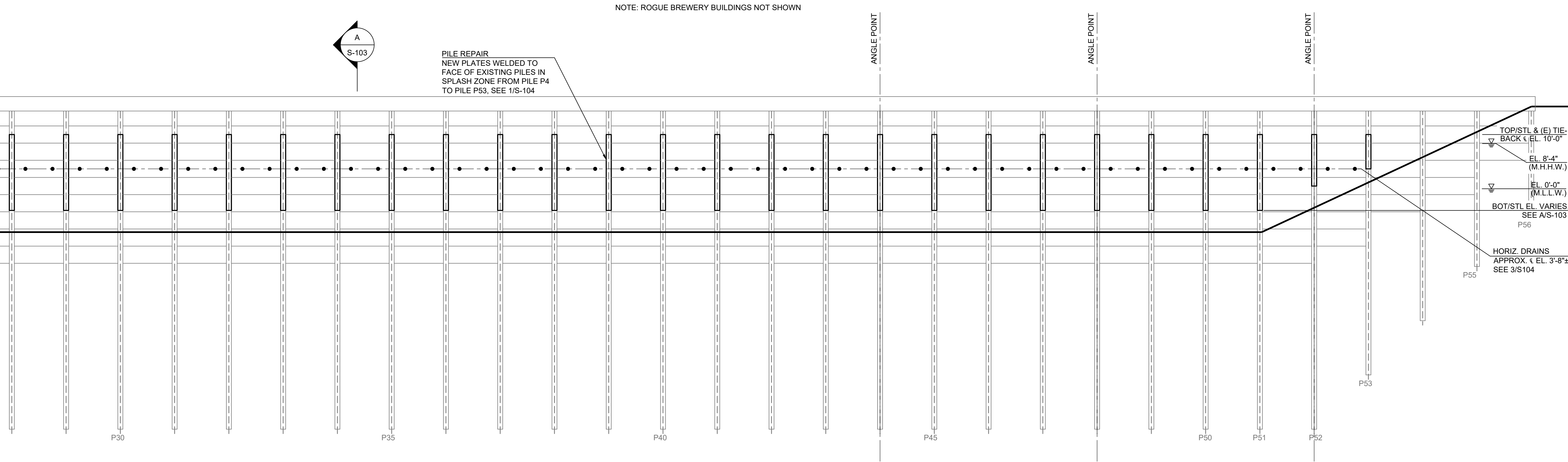
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----- MATCHLINE - SEE SHEET S-102A -----



1
S-101

ENLARGED SEAWALL ELEVATION - AREA B
SCALE: 1" = 10'

TIDAL DATA		
ELEVATION	M.L.L.W. (FT)	NAVD88 (FT)
BASE FLOOD	13.74	13.00
H.M.T.	12.25	11.51
M.H.H.W.	8.34	7.60
M.H.W.	7.65	6.91
M.T.L.	4.52	3.78
M.S.L.	4.46	3.72
D.T.L.	4.17	3.43
M.L.W.	1.38	0.64
NAVD88	0.74	0.00
M.L.L.W.	0.00	-0.74
MIN.	-3.52	-4.26

BASE FLOOD	100-YEAR FLOOD
H.M.T.	HIGHEST MEASURED TIDE (12-11-1969)
M.H.H.W.	MEAN HIGHER-HIGH WATER
M.H.W.	MEAN HIGH WATER
M.T.L.	MEAN TIDE LEVEL
M.S.L.	MEAN SEA LEVEL
D.T.L.	MEAN DIURNAL TIDE LEVEL
M.L.W.	MEAN LOW WATER
NAVD88	NORTH AMERICAN VERTICAL DATUM OF 1988
M.L.L.W.	MEAN LOWER-LOW WATER
MIN.	LOWEST OBSERVED WATER LEVEL (06-01-1973)

EXISTING PILING APPROXIMATE DATA		
PILE	TIP ELEV	LENGTH
1 & 55	-14'-4"	30'
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5 & 53	-34'-4"	50'
6	-39'-4"	55'
7 TO 52 INCL.	-44'-4"	60'
56	-4'-4"	20'

100% SUBMITTAL



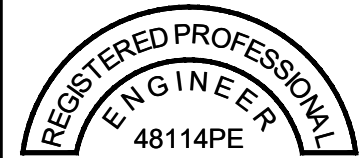
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ENLARGED SEAWALL ELEVATION - AREA B FOR:
ROGUE BREWERY SEAWALL REPAIR
A SITE LOCATED IN THE CITY OF NEWPORT, OREGON



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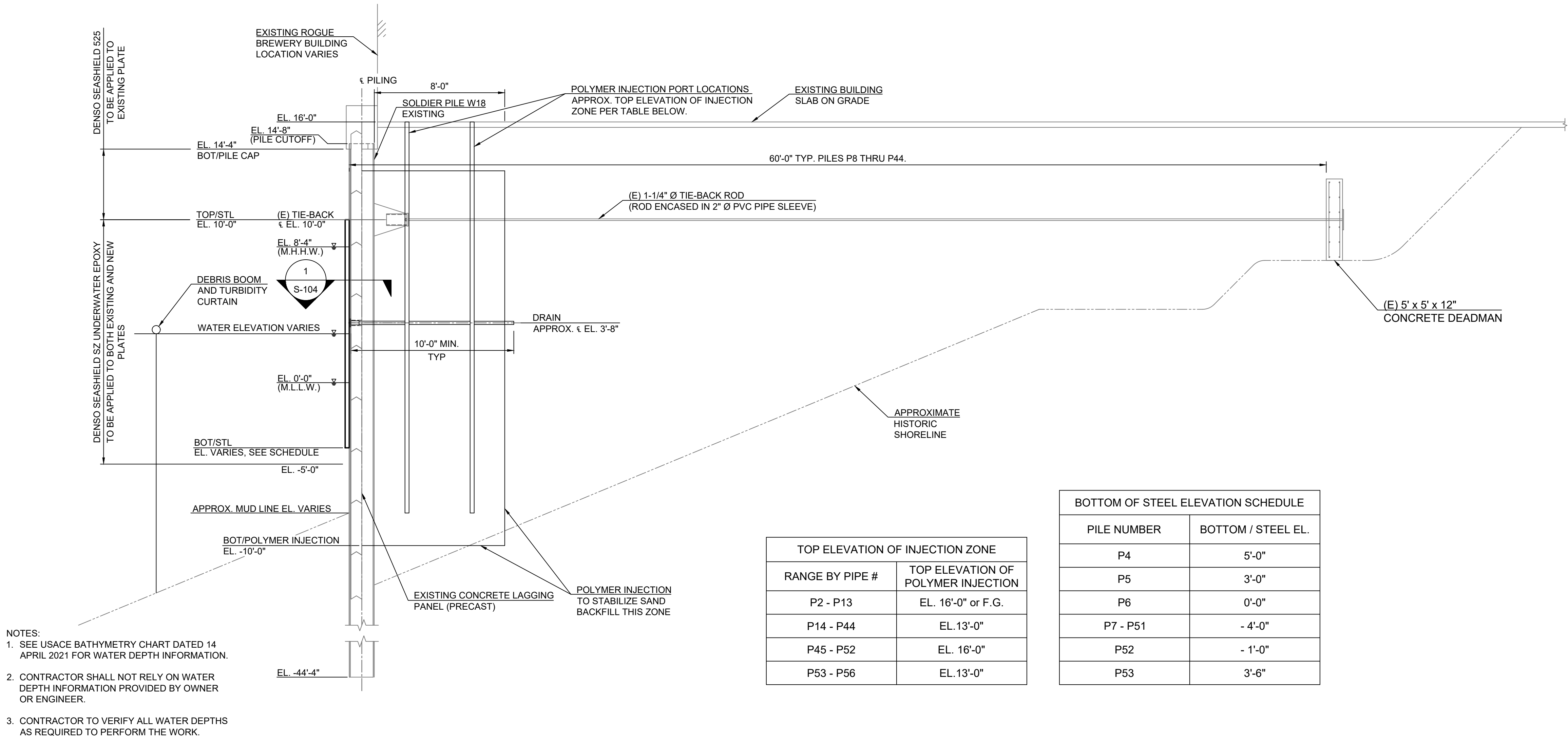
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S-102B

SHEET 9 OF 11

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- NOTE:
1. NEW GUIDE PILE CONNECTIONS PER 2/S104



TOP ELEVATION OF INJECTION ZONE	
RANGE BY PIPE #	TOP ELEVATION OF POLYMER INJECTION
P2 - P13	EL. 16'-0" or F.G.
P14 - P44	EL. 13'-0"
P45 - P52	EL. 16'-0"
P53 - P56	EL. 13'-0"

BOTTOM OF STEEL ELEVATION SCHEDULE	
PILE NUMBER	BOTTOM / STEEL EL.
P4	5'-0"
P5	3'-0"
P6	0'-0"
P7 - P51	- 4'-0"
P52	- 1'-0"
P53	3'-6"

SEAWALL SECTION VIEW
SCALE: 1/4" = 1'-0"

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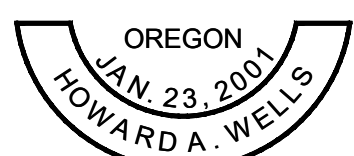
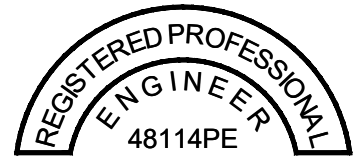
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SEAWALL SECTION VIEW FOR:
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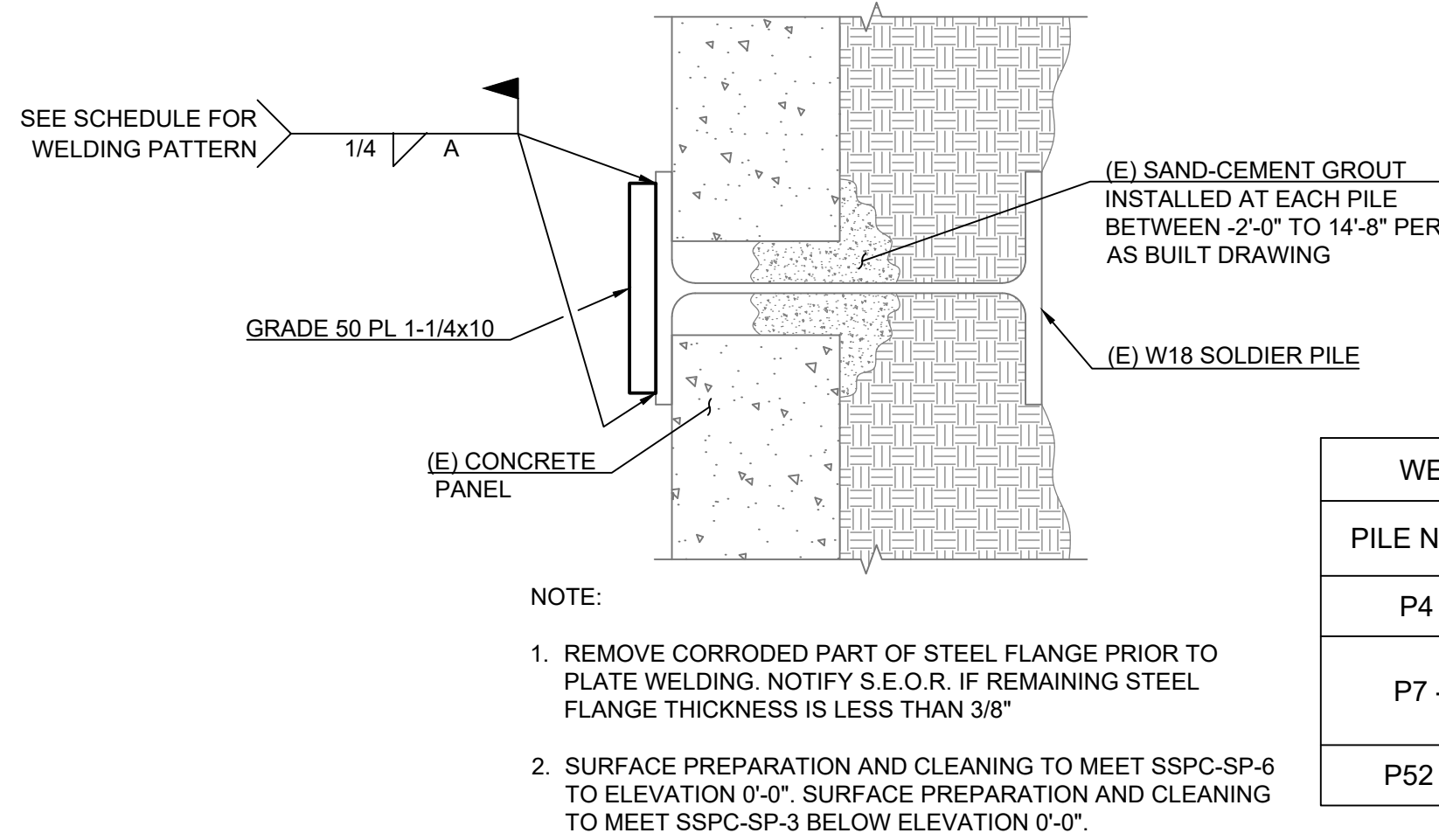
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S-103

SHEET 10 OF 11

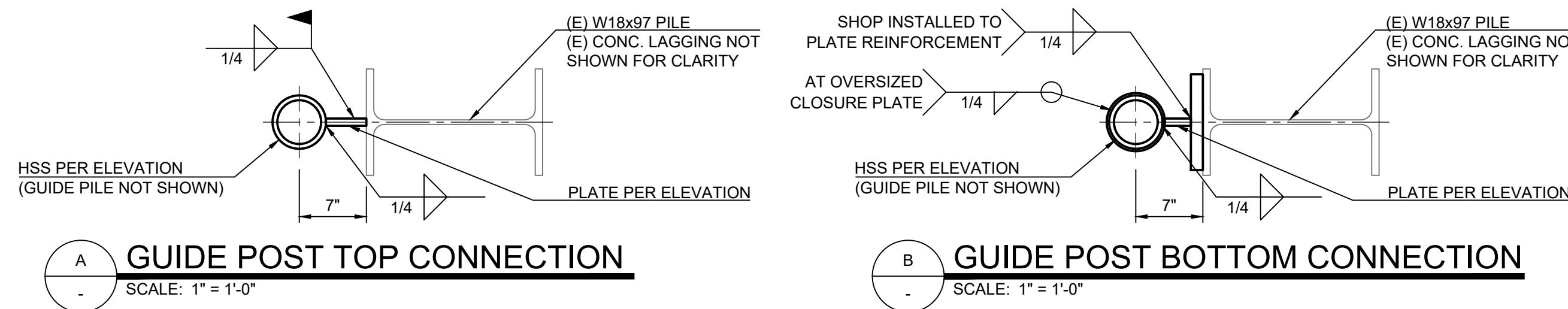
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WELDING PATTERN SCHEDULE	
PILE NUMBER	"A"
P4 - P6	5-12
P7 - P51	8-12 TOP 4' 3-12 BOTTOM 4' 5-12 REST OF LENGTH
P52 - P53	5-12

1 STEEL PLATE REINFORCEMENT TO EXISTING SOLDIER PILE FLANGE

SCALE: 1-1/2" = 1'-0"

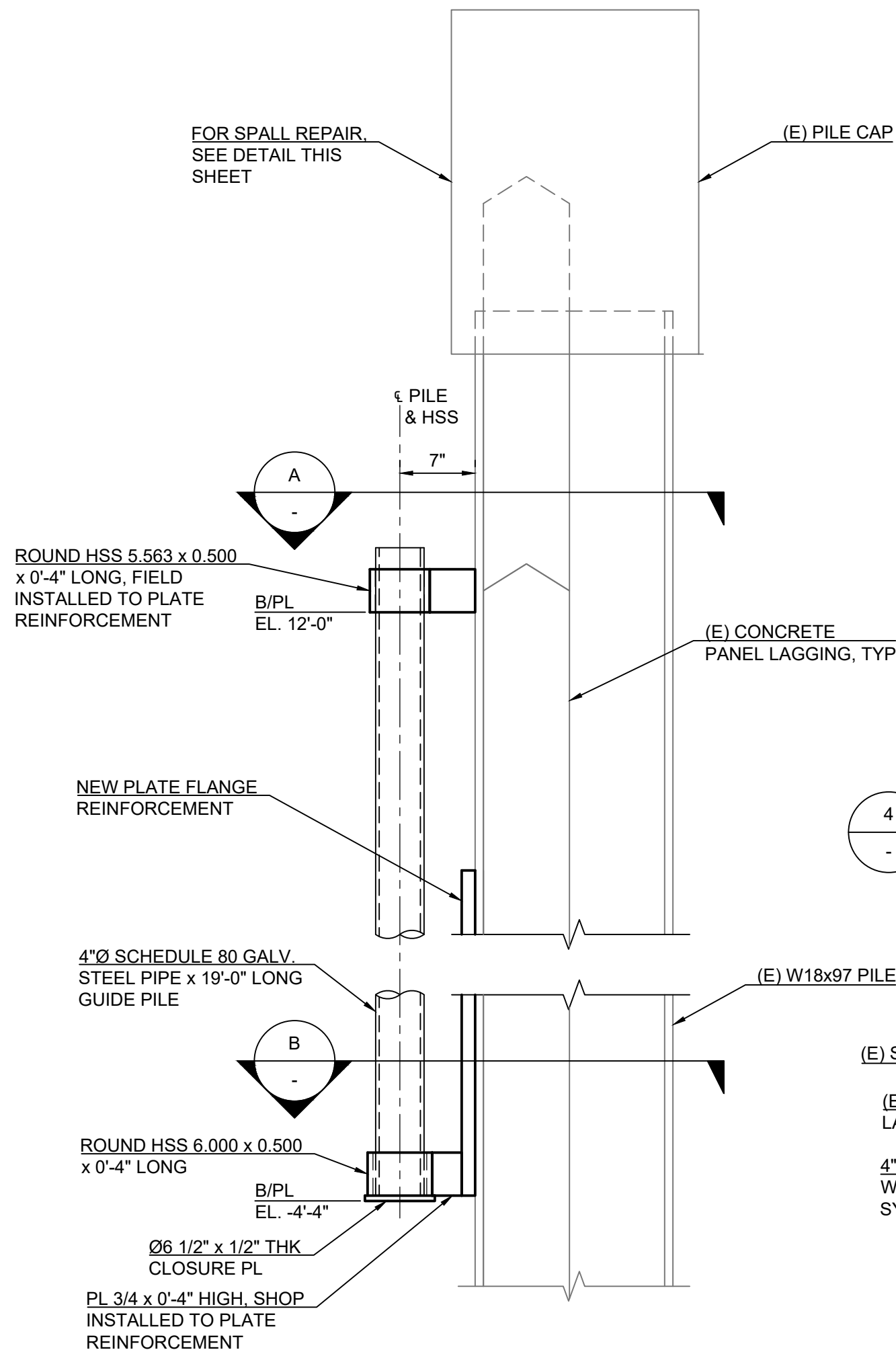


A GUIDE POST TOP CONNECTION

SCALE: 1" = 1'-0"

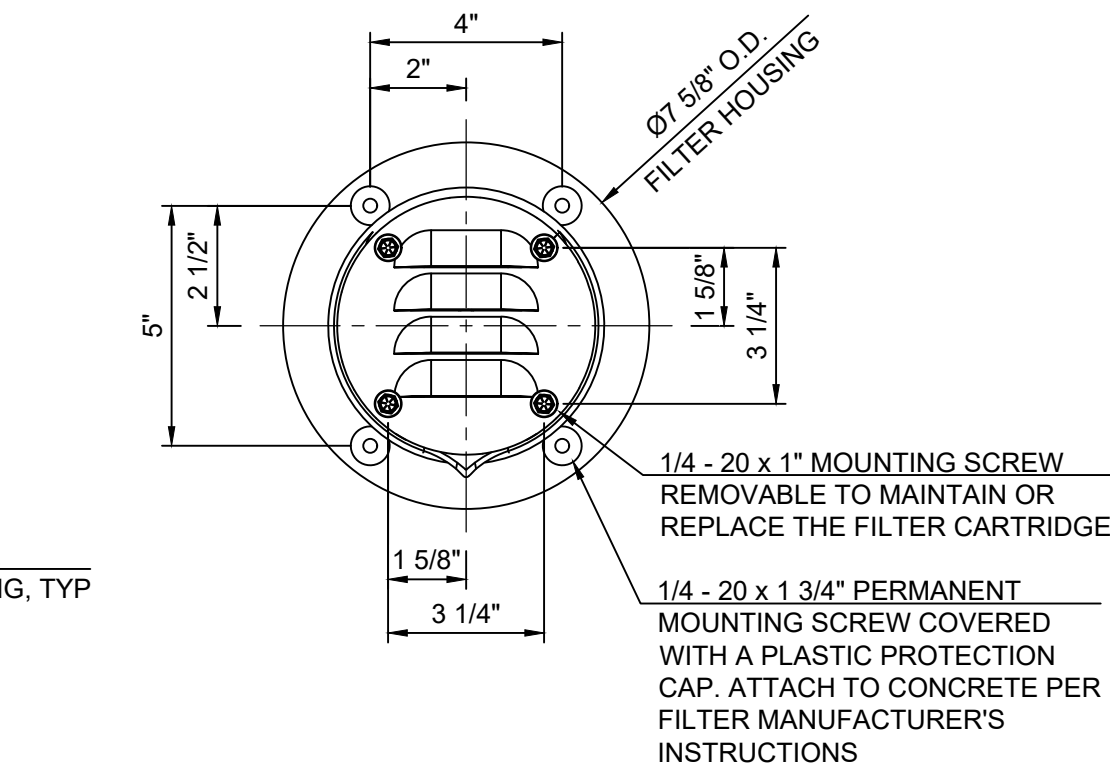
B GUIDE POST BOTTOM CONNECTION

SCALE: 1" = 1'-0"



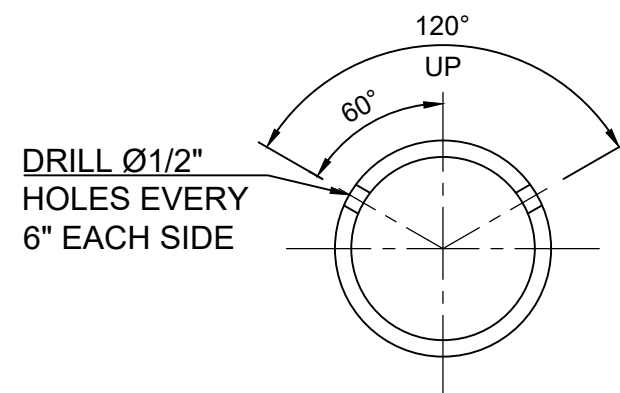
2 GUIDE POST ELEVATION

SCALE: 1" = 1'-0"



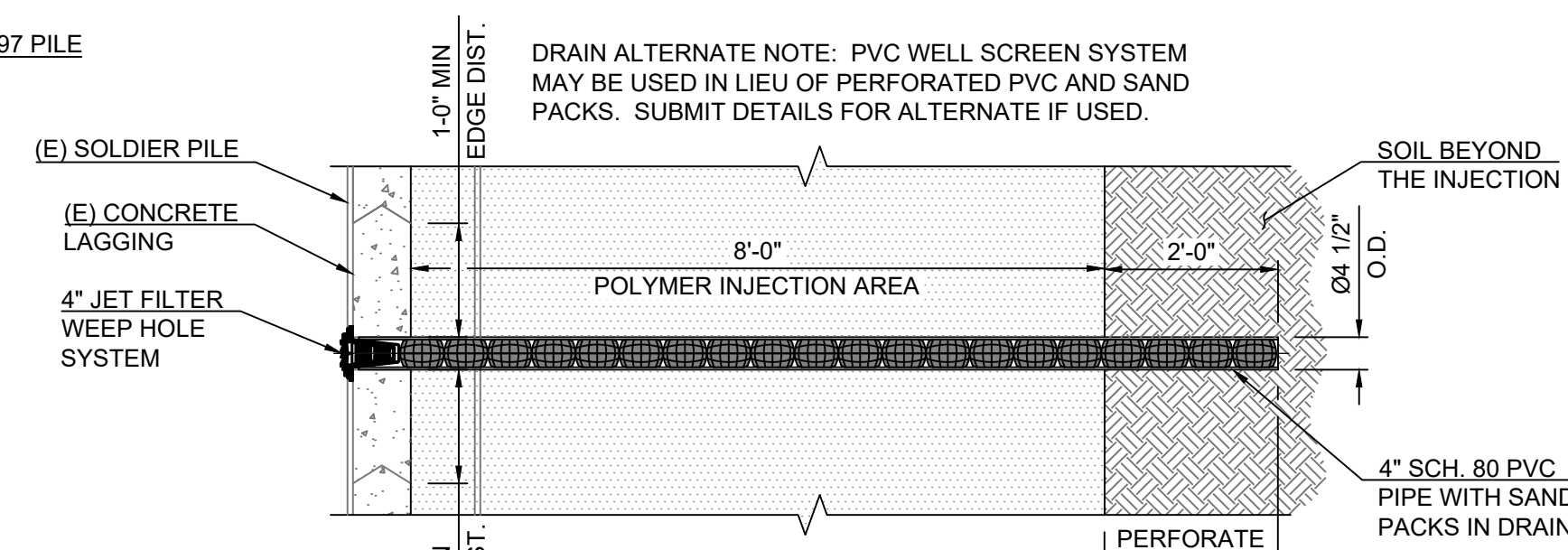
4 JET FILTER WEEP HOLE SYSTEM

SCALE: 3" = 1'-0"



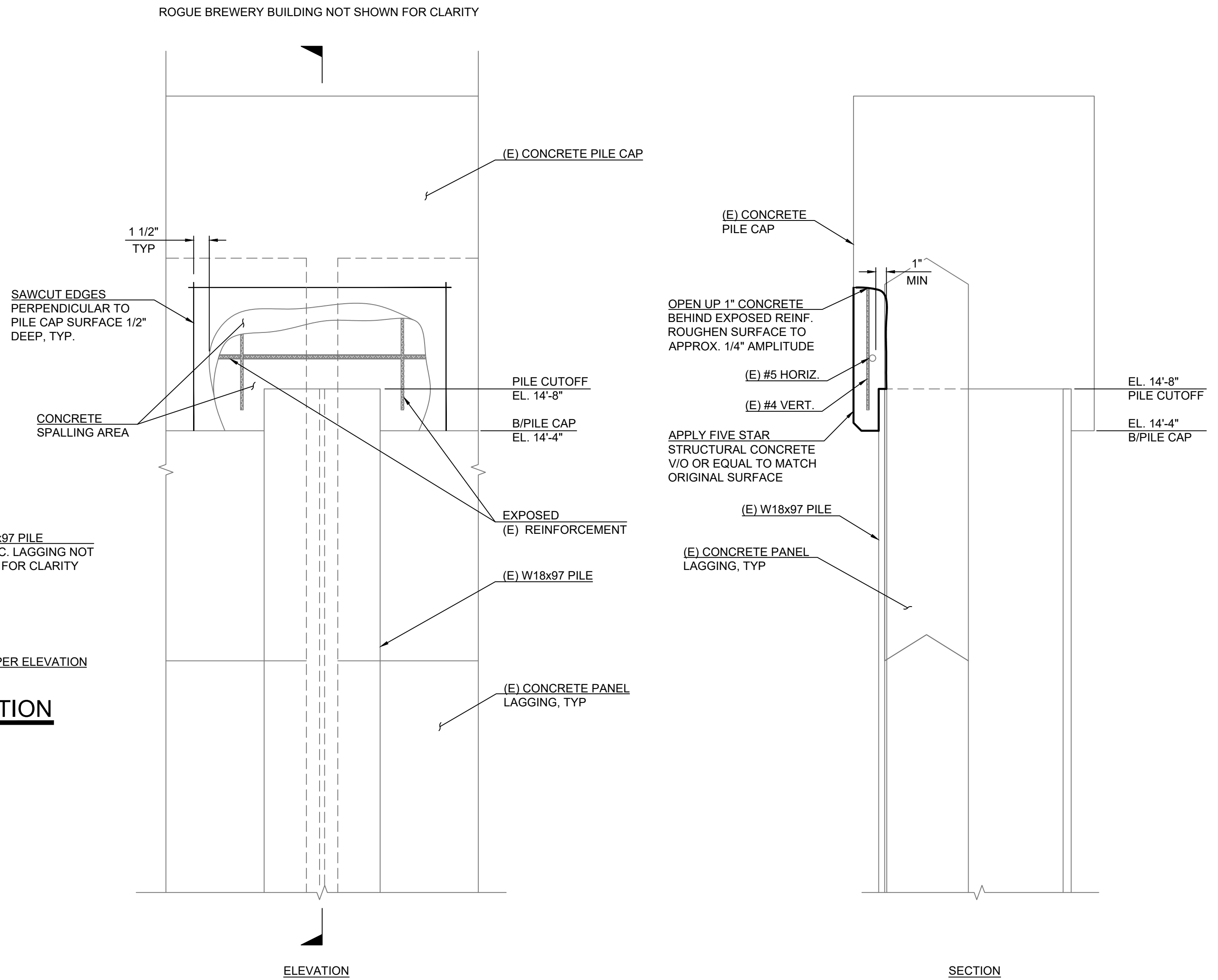
5 PIPE PERFORATION

SCALE: 3" = 1'-0"



B DRAIN SECTION

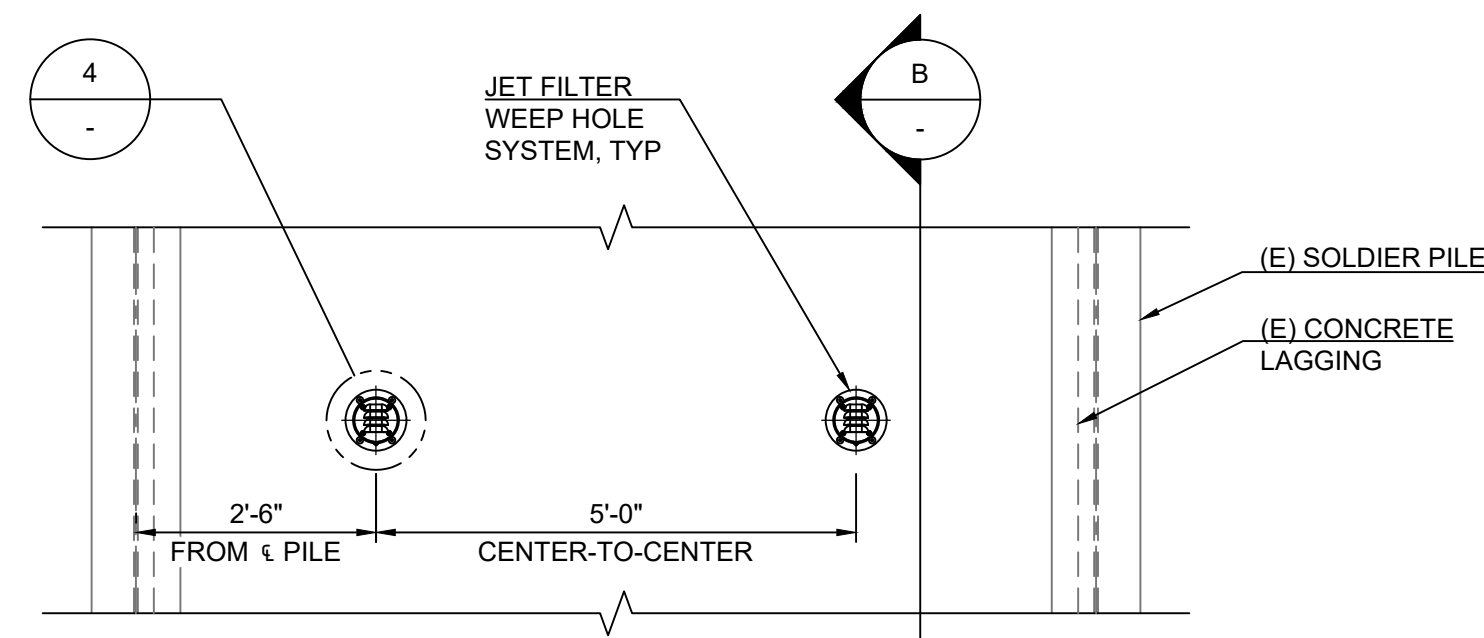
SCALE: 1/2" = 1'-0"



- NOTE:
- CLEAR COVER TO EXISTING REINFORCEMENT IS 1 1/4" PER AS BUILT DRAWING.
 - DO NOT CUT EXISTING REINFORCEMENT.
 - SURFACE PREPARATION & CLEANING REQUIREMENTS PRIOR TO NEW GROUT INSTALLATION:
 - CONCRETE SHOULD BE CLEANED OF DUST, CONTAMINATES AND FRACTURED CONCRETE FOR BONDING.
 - RUST AND CEMENT PASTE MUST BE REMOVED FROM EXISTING REBAR
 - CONCRETE SURFACE SHOULD BE SATURATED SURFACE DRY
 - CONTINUE CONCRETE REMOVAL TO REACH 3/4" MIN. CLEARANCE BEHIND EXPOSED REINFORCEMENT.
 - CLEAN EXCESSIVE CORROSION ON SOLDIER PILE SURFACE AND APPLY COATING
 - COATING TO BE APPLIED TO BOTH NEW AND EXISTING REINFORCEMENT.
 - REFER TO ACI RAP BULLETIN 6 FOR DETAILED PROCEDURE.
 - SUBMIT WORKPLAN FOR SEOR REVIEW PRIOR TO CONSTRUCTION.

CONCRETE PILE CAP SPALL REPAIR

SCALE: 1-1/2" = 1'-0"



3 DRAIN DETAIL

SCALE: 1/2" = 1'-0"

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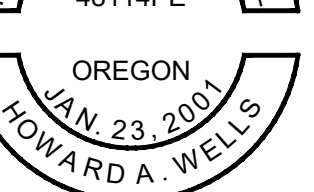
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DETAILS FOR:
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